

Miscellaneous Capacity and Safety Improvement Evaluations

SR 434 at McCulloch Road Median Closure Technical Memorandum

Seminole County, Florida

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Section 1

Introduction

INTRODUCTION

Kittelson & Associates, Inc. was retained by Seminole County to conduct a traffic operations evaluation of the Private Roadway intersection with McCulloch Road approximately 330 feet west of SR 434. The primary purpose of this study is to evaluate operations and safety concerns with the eastbound approach of McCulloch Road at SR 434 where typical queues extend past the left turn storage and the private road intersection. Analysis includes the signalized intersection's interaction with the nearby private road intersection and evaluates impacts of closing the median.

The current geometry of the private road intersection is full access, allowing left turns for all approaches, in and out of the private roadways. However, the proximity of the signalized intersection on McCulloch Road at SR 434 and this unsignalized intersection creates challenges for peak hour traffic operations, driver expectancy, and intersection safety – often resulting in an eastbound queue on McCulloch Road that does not clear during the evening peak. The existing unsignalized median opening was evaluated for a median closure, restricting access for the driveways to only right turns in and out.

PROJECT DESCRIPTION

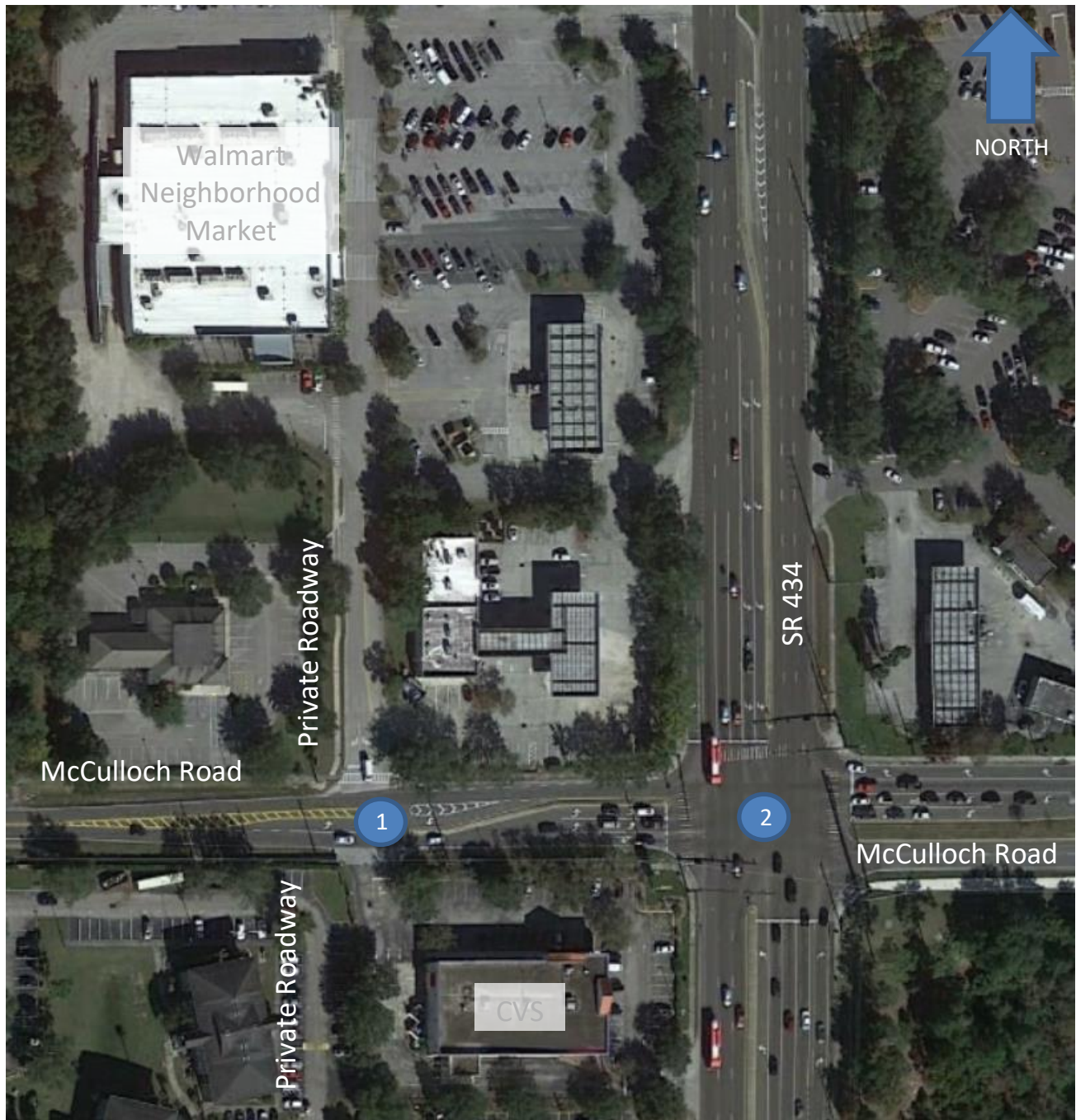
The intersection of McCulloch Road at SR 434 is located in Seminole County, Florida near the University of Central Florida. McCulloch Road lies along the county line of Orange and Seminole counties. McCulloch Road is classified as an urban collector and SR 434 is classified as an urban principal arterial. This signalized intersection operates using adaptive signal control. Driveways to CVS and a Walmart Neighborhood Market lie just to the west of the signalized intersection and have full access for left turns, through movements and right turns via a median opening.

There are two study intersections:

1. McCulloch Road at Unsignalized Full Median Opening (330 feet west of SR 434)
2. McCulloch Road at SR 434 (Signalized)

Figure 1 illustrates the project area and study intersections.

Figure 1: Study Area



Section 2

Data Collection

DATA COLLECTION

TURNING MOVEMENT COUNTS

Data collection was conducted to develop turning movement volumes at both study intersections. Counts were collected on Tuesday, September 8, 2022, during the AM (7-9 AM) and PM (4-6 PM) peak periods. Raw turning movement counts are provided in **Appendix A**.

SIGNAL TIMING

Current signal timing plans, provided in **Appendix B**, were obtained from Orange County for the signalized intersection of McCulloch Road and SR 434.

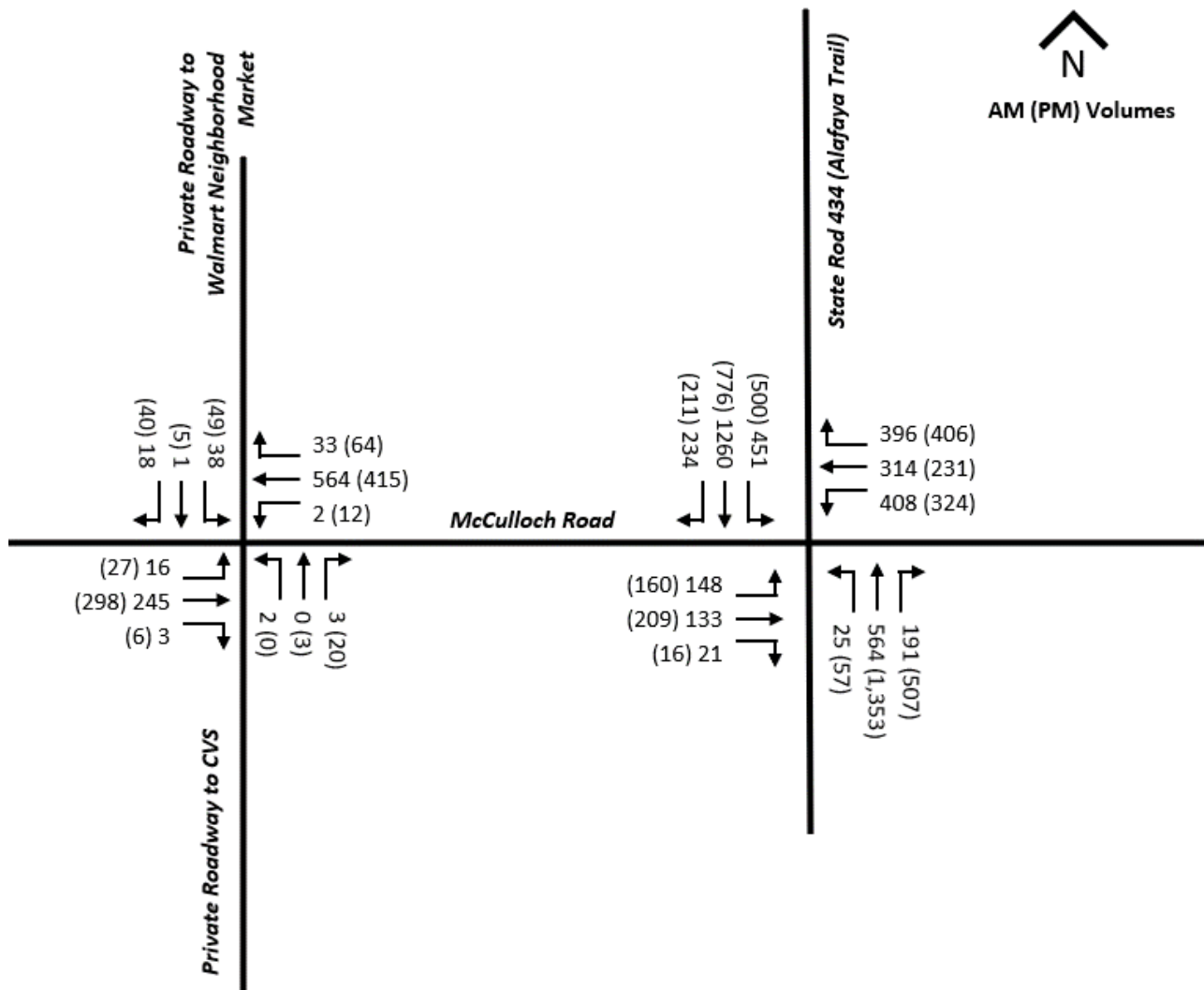
Section 3

Volume Development

VOLUME DEVELOPMENT

Traffic volumes were developed to be used in the operational analysis. Intersection turning volumes collected at both intersections in September 2022 were adjusted using a seasonal factor of 1.03. No growth factors were applied. These adjusted existing (2022) AM and PM peak hour intersection volume estimates are illustrated in **Figure 2**. The seasonal factor report is provided in **Appendix C**.

Figure 2: Existing 2022 AM and PM Peak Hour Intersection Volumes



Section 4

Existing Conditions Analysis

EXISTING CONDITIONS ANALYSIS

The purpose of the existing conditions analysis is to gain an understanding of the study area's traffic operations and safety performance to identify existing deficiencies and inform potential improvement alternatives. This section describes issues and opportunities observed through field observations, existing intersection operations analysis, and historical crash analysis.

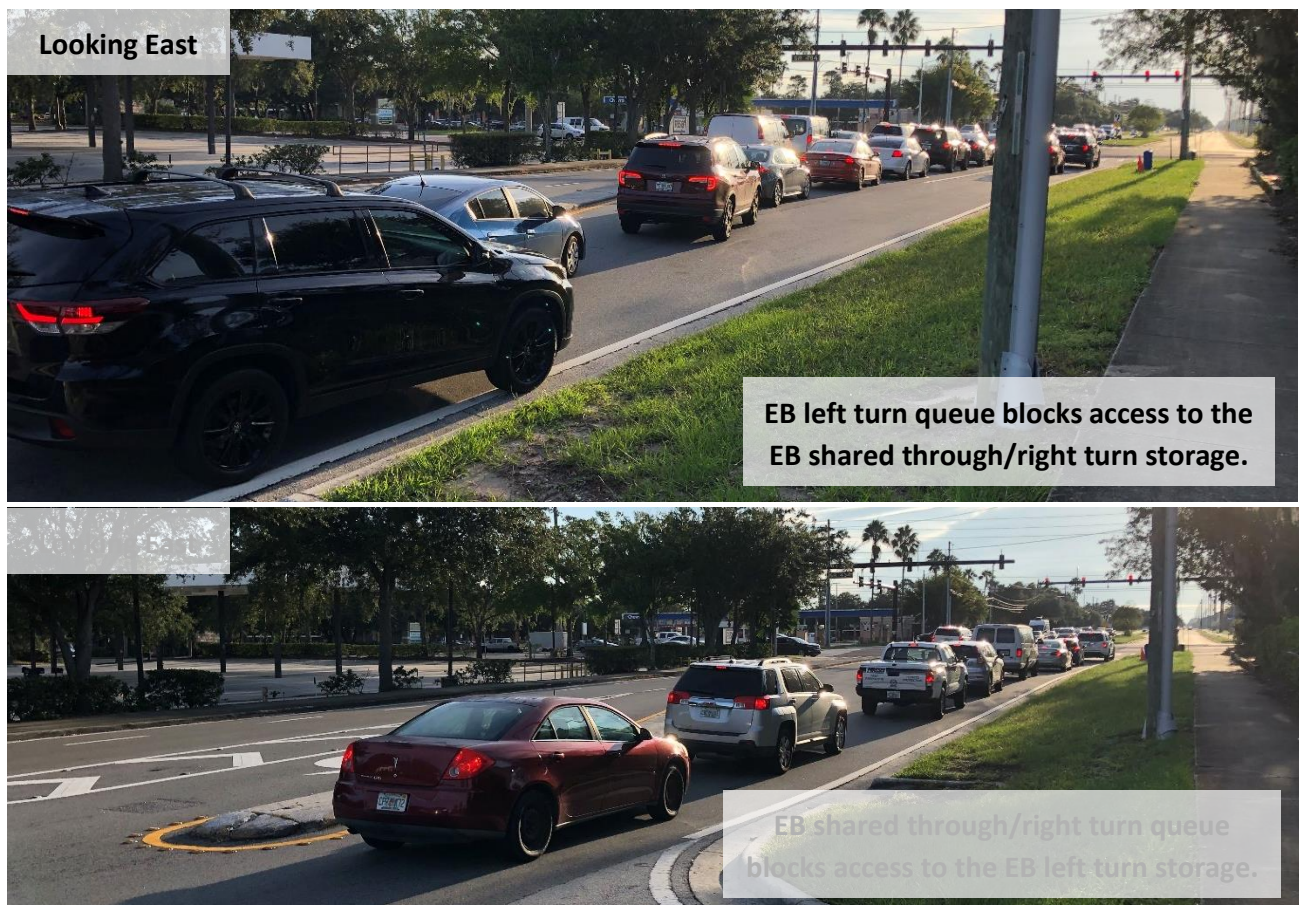
FIELD OBSERVATIONS

Field reviews were conducted during the AM and PM peak hours on Wednesday, September 14, 2022, and Thursday, September 15, 2022, to observe traffic operations at the intersection of McCulloch Road and SR 434 and the surrounding study area. The following are observations from these field reviews.

AM Peak Period

- The queue for the eastbound approach often blocked access to either the left turn storage or the through lane (see **Figure 3**).

Figure 3: Eastbound Traffic Queueing at McCulloch Road and SR 434, AM



- The eastbound approach cleared during most cycles.
- Vehicles turning out of the northbound and southbound private roadway approaches through the median opening sometimes halted the flow of eastbound traffic, causing cycle failures.

PM Peak Period

- The queue for the eastbound approach often blocked the left turn storage (see **Figure 4**).

Figure 4: Eastbound Traffic Queueing at McCulloch Road and SR 434, PM



- Eastbound queues were observed extending through the median opening (see **Figure 5**).

Figure 5: Eastbound Traffic Queueing at McCulloch Road and SR 434, PM



- The eastbound approach failed to clear for several cycles during the PM peak hour.
- Vehicles turning out of the northbound and southbound private roadway approaches through the median opening oftentimes halted the flow of eastbound traffic, causing cycle failures.
- Toward the end of the green time, vehicles were observed accelerating hard to make it through the intersection.

- During one cycle when the eastbound left turn storage was blocked by the queue (see **Figure 6**), a vehicle was observed bypassing the queue by driving over the concrete traffic separator into the left turn storage.

Figure 6: Queue Blocks Left-Turn Storage on Eastbound McCulloch Road at SR 434, PM



EXISTING INTERSECTION OPERATIONS ANALYSIS

The existing intersection operating conditions (2022) were evaluated for the weekday AM and PM peak hour traffic volume conditions. The intersection level of service (LOS) was analyzed using *Highway Capacity Manual (HCM) 6th Edition* methodologies as implemented by Synchro Version 11. **Table 1** and **Table 2** summarize the existing AM and PM peak hour intersection operations. For the unsignalized, two-way stop-controlled intersection, average delay and LOS are presented for the critical movement on each approach. For the signalized intersection, average delay and LOS are presented for each movement and for the overall intersection. The volume-to-capacity (V/C) ratios are reported for the critical movement on each approach. Detailed HCM output reports are provided in **Appendix D**.

Table 1: Existing 2022 Intersection Operations Analysis, AM Peak Hour

Intersection	Control	Measure	AM Peak Hour											
			Overall	EB		WB			NB			SB		
				L	T/R	L	T	R	L	T	R	L	T	R
McCulloch Road & Private Roadway	Two-Way Stop Control	LOS	-	A	-	A	-	-	A			C		B
		Delay (s)	-	9.3	-	7.8	-	-	9.7			23.9		13
		V/C	-	0.02	-	0.00	-	-	0.00			0.18		0.04
		Queue (ft)	-	25	-	25	-	-	0			25		25
McCulloch Road & SR 434	Signal	LOS	E	F	F	E	E	F	F	D	B	F	C	C
		Delay (s)	57.0	96.1	96.3	67.0	68.9	93.8	122.3	44.9	14.2	88.0	33.4	27.3
		V/C	-	0.81	0.88	0.65	0.77	0.94	0.81	0.37	0.27	0.91	0.58	0.26
		Queue (ft)	-	200	325	375	550	650	75	300	125	450	525	225

Table 2: Existing 2022 Intersection Operations Analysis, PM Peak Hour

Intersection	Control	Measure	PM Peak Hour											
			Overall	EB		WB			NB			SB		
				L	T/R	L	T	R	L	T	R	L	T	R
McCulloch Road & Private Roadway	Two-Way Stop Control	LOS	-	A	-	A	-	-	B			C		B
		Delay (s)	-	8.5	-	7.9	-	-	11.5			24.5		11.7
		V/C	-	0.03	-	0.01	-	-	0.04			0.24		0.08
		Queue (ft)	-	25	-	0	-	-	25			25		25
McCulloch Road & SR 434	Signal	LOS	E	F	F	F	E	F	E	D	E	F	C	B
		Delay (s)	67.4	110.5	109.8	104.9	73.4	99.0	78.9	54.5	58.6	97.8	33.8	17.6
		V/C	-	0.84	0.92	0.91	0.68	0.91	0.33	0.78	0.75	0.93	0.38	0.22
		Queue (ft)	-	225	500	375	425	550	125	725	650	525	350	150

All movements at the unsignalized full median opening operate at LOS C or better from a capacity standpoint, but the HCM capacity model does not account for the interaction with eastbound queues from the adjacent signalized intersection. The signalized intersection movements at McCulloch Road & SR 434 operate at LOS E or better and with v/c ratios less than 1.0, with the following exceptions:

- In the AM peak hour:
 - The eastbound left turn movement operates at LOS F with an average delay of 96 seconds.
 - The eastbound shared through and right turn lane movement operates at LOS F with an average delay of 96 seconds.
 - The westbound right turn movement operates at LOS F with an average delay of 94 seconds.
 - The northbound left turn movement operates at LOS F with an average delay of 122 seconds.
 - The southbound left turn movement operates at LOS F with an average delay of 88 seconds.
- In the PM peak hour:
 - The eastbound left turn movement operates at LOS F with an average delay of 111 seconds.
 - The eastbound shared through and right turn lane movement operates at LOS F with an average delay of 110 seconds.
 - The westbound left turn movement operates at LOS F with an average delay of 105 seconds.
 - The westbound right turn movement operates at LOS F with an average delay of 99 seconds.
 - The southbound left turn movement operates at LOS F with an average delay of 98 seconds.

HISTORICAL CRASH ANALYSIS

Crash data at the study intersections for the most recent five (5) years (2017-2021) and partial 2022 data through August 31, 2022, was obtained from FDOT's Crash Analysis Reporting (CAR) System, the State Safety Office GIS (SSOGis), and Signal Four Analytics (S4). A crash analysis summary is provided in this section. Full crash summary sheets are provided in **Appendix E**.

Crashes by Year

- A total of 297 crashes occurred at the study intersections between January 1, 2017, and August 31, 2022.
- There were sixty-three (63) crashes in 2017, sixty-one (61) crashes in 2018, fifty-seven (57) crashes in 2019, forty-four (44) crashes in 2020, forty-four (44) crashes in 2021, and twenty-eight (28) crashes through August 31, 2022.
 - No geometric or land-use changes occurred within the study area from 2017 to 2022.
 - SR 434 AADT has fluctuated year to year from 51,180 in 2017, to 51,833 in 2018, to 60,648 in 2019, to 45,552 in 2020, to 40,388 in 2021.
- There were no fatal crashes reported between January 1, 2017, and August 31, 2022.
- Forty-three (43) crashes involved possible injuries, nineteen (19) involved non-incapacitating injuries, and four (4) involved incapacitating injuries. Injury-related crashes accounted for 22% of crashes.
- Two hundred and thirty-one (231) crashes were property damage only (PDO), accounting for 78% of all crashes.
- One hundred (100) crashes, or 34%, occurred during nighttime conditions.
 - Roadway lighting is present at the SR 434 and McCulloch Road intersection. There is no lighting at the median opening on McCulloch.
- Sixty-three (63) crashes, or 22%, occurred under wet pavement conditions.
- One (1) crash involved a pedestrian. There were three (3) bicycle crashes, or 1%.
 - The pedestrian crash (Crash #248881936) occurred when a pedestrian crossing McCulloch Road on the east leg crosswalk of the SR 434 intersection was struck by a westbound right turning vehicle. The crash occurred at 6:18 PM at dusk, clear weather, and dry roadway surface conditions. The pedestrian experienced non-incapacitating injuries due to the crash. The driver was determined to be at fault for failing to yield right of way to the pedestrian at the intersection.
 - One bicyclist crash (Crash #88219029) occurred when a bicyclist crossing the CVS driveway along SR 434 was struck by an eastbound right turning vehicle coming out of the CVS driveway. The crash occurred at 10:37 AM under daylight, clear weather, and dry roadway surface conditions. The bicyclist experienced no injuries due to the crash.
 - The second bicyclist crash (Crash #88232537) occurred when a bicyclist crossing SR 434 on the north leg crosswalk of the McCulloch Road intersection was struck by a westbound right turning vehicle. The crash occurred at 1:42 PM under daylight, clear weather, and dry roadway surface conditions. The bicyclist experienced non-incapacitating injuries due to the crash. No fault was determined.

- The third bicyclist crash (Crash #88315222) occurred when a bicyclist traveling southbound along SR 434 at the east leg crosswalk of the McCulloch Road intersection was struck by a westbound right turning vehicle. The crash occurred at 2:32 PM under daylight, clear weather, and dry roadway surface conditions. The bicyclist experienced non-incapacitating injuries due to the crash.

Forty-one (41) angle and left-turn crashes were identified in the study area from historical data. The following list provides a breakdown of the angle and left turn crashes by intersection:

- Nineteen (19) angle and left turn crashes occurred at the McCulloch Road and SR 434 intersection.
- Nine (9) angle and left-turn crashes occurred at the median opening north of McCulloch Road on SR 434 (access to Walmart Neighborhood Market to the west and Publix to the east).
- Four (4) angle and left-turn crashes occurred at the median opening east of SR 434 at McCulloch Road (access to Publix and other shops to the north).
- One (1) left-turn crash occurred at the median opening south of McCulloch Road; a northbound left vehicle hit a southbound vehicle.

Crash data was examined more thoroughly at the median opening to the west of SR 434 on McCulloch Road (access to CVS to the south and Walmart Neighborhood Market to the north). There were 8 angle and left turn crashes at the median opening.

- Five (5) southbound left vehicles collided with eastbound through vehicles.
- Three (3) southbound left vehicles collided with a westbound through vehicle.

Section 5 Alternative Evaluation



ALTERNATIVE EVALUATION

One alternative was developed and evaluated to address the existing traffic operations and safety issues. This section describes the development, operations analysis, and safety screening of this potential alternative.

MEDIAN CLOSURE ALTERNATIVE DEVELOPMENT

The median opening along McCulloch Road west of SR 434 was modified to allow right-in/right-out only. The eastbound left turn storage at SR 434 and McCulloch Road was extended from 90 feet of storage and 100 feet of taper to 250 feet of storage and 300 feet of taper. The following describes the traffic volume rerouting assumptions associated with the proposed alternative:

- The existing northbound left turn volume from the Private Roadway near CVS was rerouted to make a northbound right turn and travel eastbound to the signalized intersection at McCulloch Road and SR 434, where they would proceed eastbound through, then U-turn and proceed westbound through the same signalized intersection and the median closure. See **Figure 7**.
- The existing northbound through volume from the Private Roadway near CVS was rerouted to make a northbound right turn and travel eastbound to the signalized intersection at McCulloch Road and SR 434, where they would proceed eastbound left to proceed northbound to reach the other entrance to the Walmart Neighborhood Market. See **Figure 7**.
- The existing southbound left turn volume from the Private Roadway near the Walmart Neighborhood Market was rerouted to make an eastbound right turn at the other exit of the Walmart Neighborhood Market on SR 434 to travel southbound to the signalized intersection of McCulloch Road and SR 434, where they were assigned to southbound through and left based on engineering judgement using existing traffic patterns and percentages. See **Figure 7**.
- The existing southbound through volume from the Private Roadway near the Walmart Neighborhood Market was rerouted to make an eastbound right turn at the other exit of the Walmart Neighborhood Market on SR 434 to travel southbound to the signalized intersection of McCulloch Road and SR 434, where they would go through the intersection and continue southbound to reach the other entrance to the CVS on SR 434. See **Figure 7**.
- The existing eastbound left turn volume from McCulloch Road toward the Walmart Neighborhood Market (via the Private Roadway) was rerouted through to the signalized intersection of McCulloch Road and SR 434, then travels eastbound left to proceed northbound toward the other entrance to the Walmart Neighborhood Market on SR 434. See **Figure 8**.
- The existing westbound left turn volume from McCulloch Road toward the CVS (via the Private Roadway) was rerouted at the signalized intersection of McCulloch Road and CR 434 before they got to this approach. Trips were shifted from southbound right to southbound through and from westbound through to westbound left to the other CVS entrance on SR 434. See **Figure 8**.

Figure 7: North/South Volume Re-route

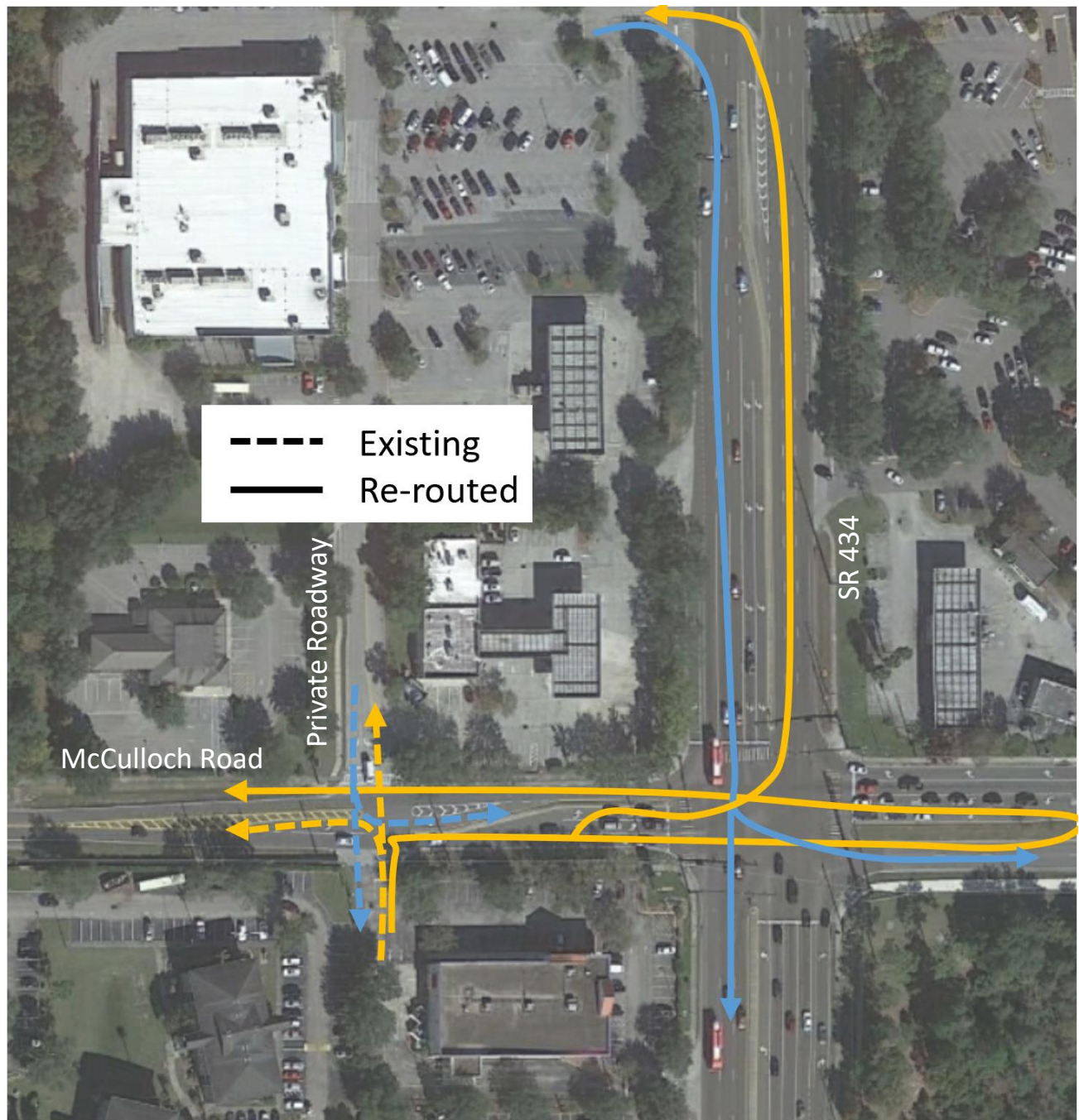
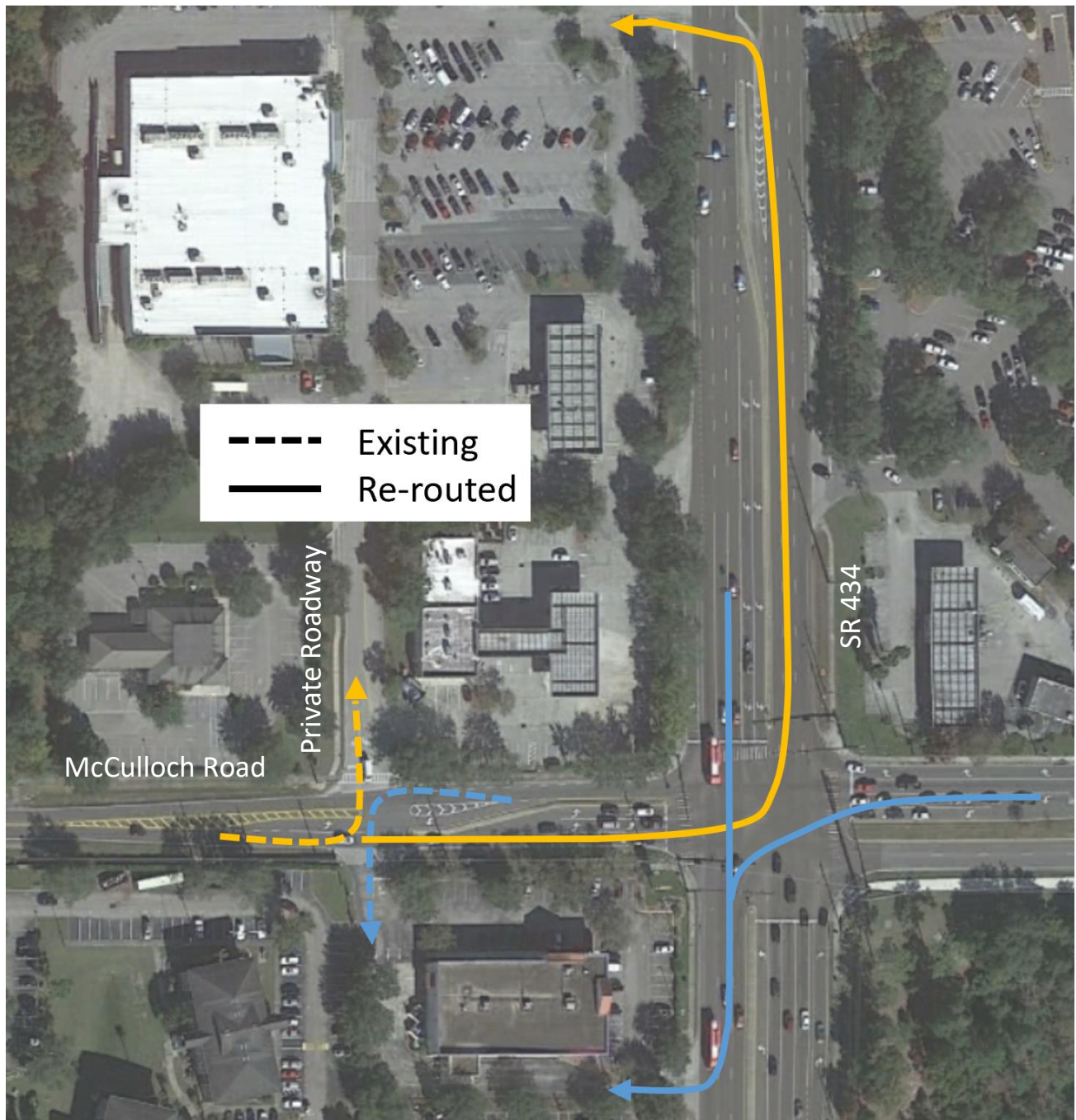
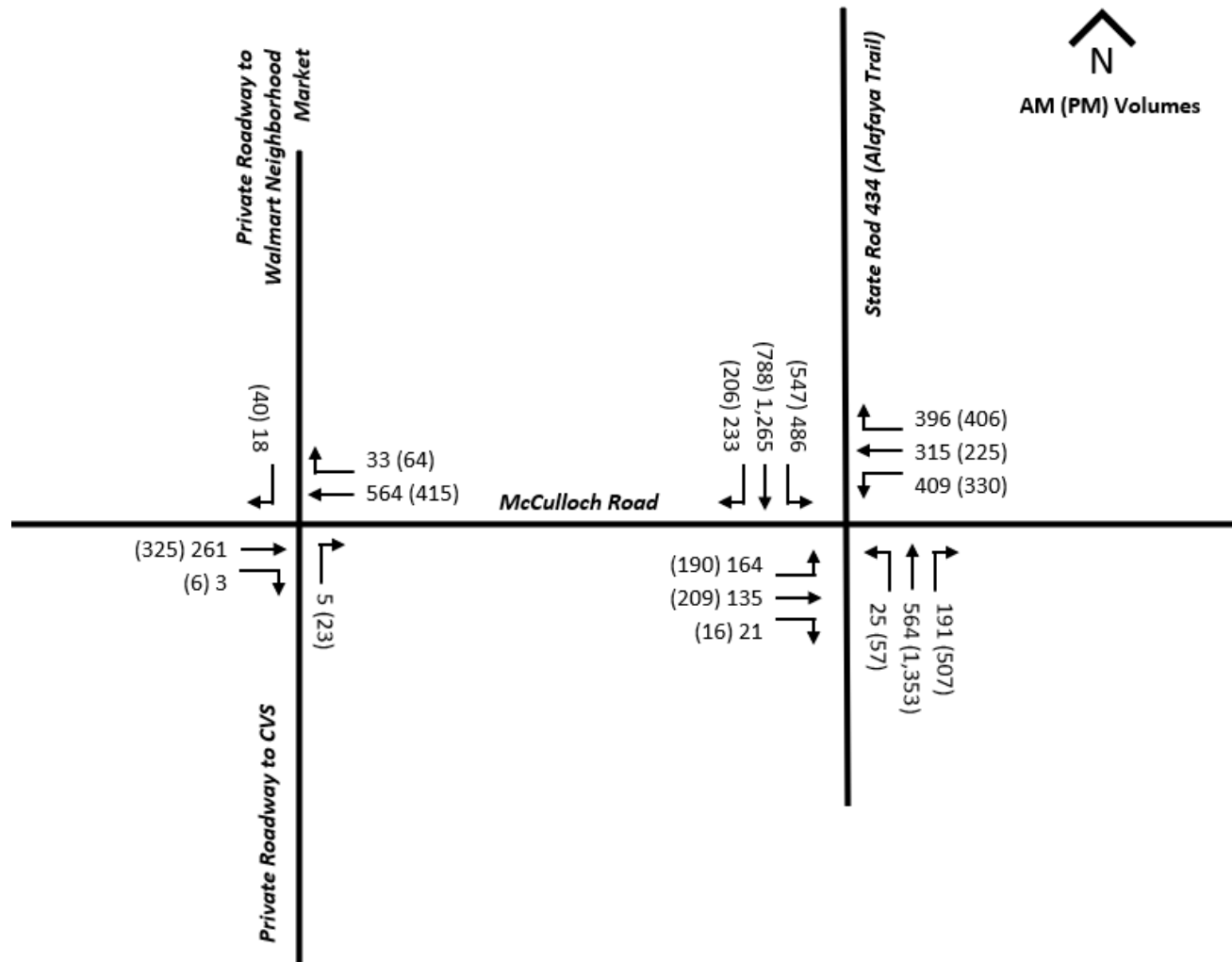


Figure 8: East/West Left Volume Re-route



After incorporating the re-routed traffic movements described above, the revised traffic volumes are illustrated in **Figure 9**.

Figure 9: Median Closure Alternative 2022 Volumes



ALTERNATIVE INTERSECTION OPERATIONS ANALYSIS

The purpose of the alternative intersection operations analysis is to gain an understanding of how the alternative performs in comparison to the existing condition.

Capacity Analysis

The 2022 intersection operating conditions for the Closed Median Alternative were evaluated for weekday AM and PM peak hour traffic volume conditions. Existing cycle lengths were maintained at the signalized intersection. In addition to the geometry improvements described in the previous section, minor signal timing improvements to the splits were made to the existing timings due to the rerouting of some traffic volume. The intersections were analyzed using HCM methodologies as implemented by Synchro Version 11 and summarized similarly to the Existing Conditions. **Table 3** and **Table 4** summarize the AM and PM peak hour intersection operations for the median closure alternative. Detailed HCM output reports are provided in **Appendix F**.

Table 3: Median Closure Alternative 2022 Intersection Operations Analysis, AM Peak Hour

Intersection	Control	Measure	AM Peak Hour											
			Overall	EB		WB			NB			SB		
				L	T/R	L	T	R	L	T	R	L	T	R
McCulloch Road & Private Roadway	Two-Way Stop Control	LOS	-		-		-	-			A			B
		Delay (s)	-		-		-	-			9.8			13.0
		V/C	-		-		-	-			0.01			0.04
		Queue (ft)	-		-		-	-			0			25
McCulloch Road & SR 434	Signal	LOS	E	F	F	E	F	E	F	D	B	F	C	C
		Delay (s)	55.9	97.5	98.2	76.9	88.0	73.1	122.3	41.9	15.0	87.4	29.1	23.7
		V/C	-	0.83	0.88	0.78	0.91	0.77	0.81	0.35	0.26	0.92	0.55	0.24
		Queue (ft)	-	200	350	400	600	425	75	275	125	475	500	200

Table 4: Median Closure Alternative 2022 Intersection Operations Analysis, PM Peak Hour

Intersection	Control	Measure	PM Peak Hour											
			Overall	EB		WB			NB			SB		
				L	T/R	L	T	R	L	T	R	L	T	R
McCulloch Road & Private Roadway	Two-Way Stop Control	LOS	-		-		-	-			B			B
		Delay (s)	-		-		-	-			10.4			11.7
		V/C	-		-		-	-			0.04			0.08
		Queue (ft)	-		-		-	-			25			25
McCulloch Road & SR 434	Signal	LOS	E	F	F	F	E	F	E	E	E	F	C	B
		Delay (s)	70.2	114.4	104.3	105.0	73.6	102.8	79.3	59.4	64.6	97.5	34.3	17.0
		V/C	-	0.87	0.90	0.91	0.68	0.93	0.34	0.83	0.79	0.94	0.39	0.21
		Queue (ft)	-	250	500	375	425	550	125	750	675	575	350	150

With the Closed Median Alternative in place, all movements at the unsignalized median opening operate at the same LOS as the existing condition, from a capacity standpoint. At McCulloch Road and SR 434 all movements in the Alternative condition operate at the same LOS as the existing condition, with the following exceptions:

- In the AM peak hour:
 - The westbound through movement operates worse, at LOS F, with average delay of 88 seconds, compared to LOS E (69 seconds of delay) in the existing condition.
- In the PM peak hour:
 - The northbound through movement operates worse, at LOS E, with average delay of 59 seconds, compared to LOS D (55 seconds of delay) in the existing condition.

The median closure affects traffic operations from a capacity perspective as explained above, but all movements remain under capacity. In addition to evaluating the capacity impacts, queue storage impacts were evaluated as discussed in the following section.

Queue Storage Evaluation

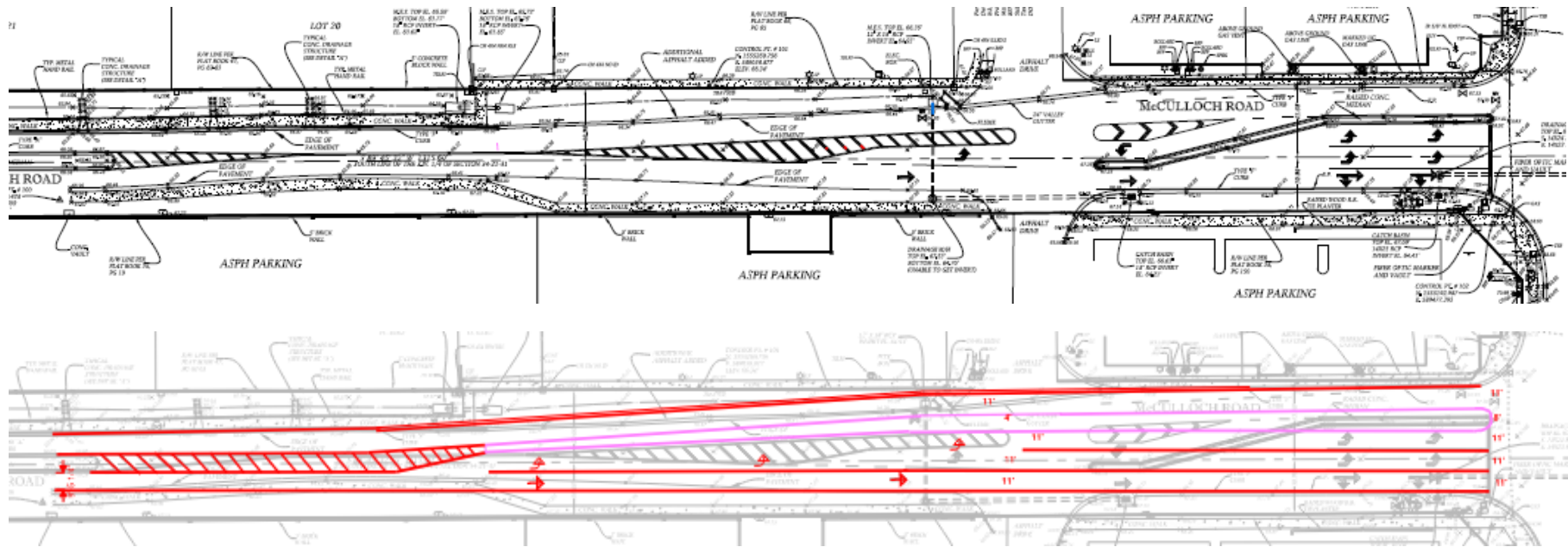
The existing eastbound approach at SR 434 and McCulloch Road includes dual left turn lanes and a shared through/right turn lane. The existing condition provides 90 feet of storage for each left turn lane and 100 feet of taper. Based on the 95th percentile queue in the existing conditions analysis, the eastbound left turn needs 225 feet (PM peak hour) of storage for both turn lanes. Field observations confirmed that the existing queues extend past the dedicated storage and the median opening during both AM and PM peak hours. With the proposed median closure and re-routed traffic volumes, the left-turn queue will require 250 feet (PM peak hour) of storage for both turn lanes. **Figure 10** depicts a planning level illustration of the proposed reconfiguration at the study intersections, which allows 250 feet of storage and another 300 feet of taper. **Table 5** describes the existing and proposed queue storage in comparison to what is needed.

Table 5: SR 434 and McCulloch Rd Eastbound Approach Taper and Queue Storage

Scenario	Taper (ft)	Queue Storage (ft)	Needed (ft)
Existing	100	90	225
Alternative	300	250	250

The Closed Median Alternative restricts access to right in/right out and extends the queue storage space which provides better access to the queue storage. With better access to storage, the eastbound movements will be able to better utilize their allotted green time.

Figure 10: Existing Median Opening and Median Closure Alternative



SAFETY SCREENING

The median closure alternative was evaluated for safety impacts related to the improvement. A qualitative evaluation was conducted based on field observations and engineering judgement. The following describes the safety benefits of the proposed alternative:

- Improvement: The median opening along McCulloch Road, 330 feet west of SR 434, was modified to allow right-in/right-out only.
 - Safety Benefit: The restriction of access to only right turns into and out of the private roadways reduces conflicts by eliminating the ability to turn left across traffic in both directions. The eight crashes observed at the median opening from 2017 to 2021 were by drivers making southbound left-turn maneuvers, which will no longer be allowed. It is anticipated that crashes of this type will be eliminated.
 - A review of the Crash Modification Factor (CMF) Clearinghouse was conducted to determine if research has been done that may help evaluate the crash implications of closing the median opening. Two CMFs were found that are similar to the modifications proposed by the Median Closure Alternative but are not an exact match, and so should not be directly applied.
 - CMF 2219 – Install Raised Median
 - This CMF is intended to be applied on roadway segments, which would include median closures along a segment leading to a reduction in crashes. This is like the proposed improvement as it closes a full median opening.
 - A CMF value of 0.29 is provided for all crash types, which means crashes may be reduced by 71 percent with the associated improvement.
 - CMF 9821 – Install Right-in-Right-out (RIRO) Operations at Stop-Controlled Intersections
 - This CMF is intended to be applied when closing a median opening at a three-leg intersection. This is like the proposed improvement, but the study location is a four-leg intersection.
 - A CMF value of 0.55 is provided for all crash types, which means crashes may be reduced by 45 percent with the associated improvement.
 - While neither CMF 2219 or 9821 should be directly applied to the study intersection as they do not match the proposed alternative, both provide similar scenarios and therefore can be used to qualitatively show a reduction in crashes can be expected following the median closure.
- Improvement: Left-turn storage capacity was increased and access to the storage was improved for the eastbound approach at SR 434 and McCulloch Road.
 - Safety Benefit: In the current scenario, storage is often blocked by the raised median and the shared through/right lane traffic queue spilling back. With the left turn storage extended, vehicles will be able to properly store prior to green time and will not need to accelerate as heavily to make it through the intersection. Risk of red-light-running and median jumping will be reduced.



SUMMARY

Kittelson & Associates, Inc. was retained by Seminole County Public Works Department to conduct a traffic operations and safety evaluation of McCulloch Road's intersections at SR 434 and the unsignalized full median opening that is located 330 feet west of SR 434. The proximity of these intersections and the current median structure presents challenges for peak hour traffic operations, driver expectancy, and intersection safety – often resulting in an eastbound queue on McCulloch Road backing up from SR 434 past the median opening. The primary purpose of this study is to evaluate operations and safety concerns with the eastbound approach of McCulloch Road at SR 434 and the interaction with the adjacent median opening.

An alternative geometry was proposed to address the existing operations and safety concerns. In this alternative, the median opening is closed such that access to the private roadways is restricted to right-in/right-out. The median closure alternative provides similar operational results from a capacity perspective when compared to the existing condition. Closing the median allows the eastbound left turn storage to be increased, accommodating the storage needs based on the re-routed volumes. The addition of left turn storage would improve operations and safety for traffic on the eastbound approach, allowing for better utilization of green time. The median closure also provides safety benefits by reducing conflicts at the median opening, which may reduce crashes.

It is recommended that Seminole County proceed with feasibility discussions with Orange County and the adjacent commercial properties regarding the median closure and turn lane storage adjustment.

Appendix A: Raw Count Data



Roadway Count Summary

Vanasse Hangen Brustlin, Inc.

County

Orange

City

Orlando

Intersection

CVS

& McCulloch Rd

Date

September 8, 2022

All Vehicles

VHB Project #:

63289.04

AM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
7:00 AM - 7:15 AM	0	0	0	4	0	1	5	32	1	0	83	2
7:15 AM - 7:30 AM	0	0	1	5	0	9	1	40	0	0	162	6
7:30 AM - 7:45 AM	0	0	0	6	0	5	4	35	0	3	149	3
7:45 AM - 8:00 AM	0	0	0	8	1	7	1	48	1	0	149	6
8:00 AM - 8:15 AM	0	0	0	7	0	5	6	66	0	0	157	5
8:15 AM - 8:30 AM	0	0	1	13	0	7	3	50	0	2	130	9
8:30 AM - 8:45 AM	1	0	2	8	0	3	3	55	1	0	141	6
8:45 AM - 9:00 AM	1	0	0	9	1	2	4	67	2	0	120	12
TOTAL	2	0	4	60	2	39	27	393	5	5	1,091	49
Peak Hour 7:45 AM - 8:45 AM	1	0	3	36	1	22	13	219	2	2	577	26

PM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
4:00 PM - 4:15 PM	0	1	4	15	1	17	7	76	1	3	98	17
4:15 PM - 4:30 PM	0	0	5	14	2	15	5	64	3	3	75	16
4:30 PM - 4:45 PM	0	0	1	6	2	6	7	84	2	0	84	13
4:45 PM - 5:00 PM	0	1	3	10	0	7	3	78	3	2	85	10
5:00 PM - 5:15 PM	0	2	5	6	0	11	7	76	0	1	94	9
5:15 PM - 5:30 PM	0	0	5	6	2	9	8	73	1	1	121	22
5:30 PM - 5:45 PM	0	0	4	23	2	10	9	71	1	3	97	20
5:45 PM - 6:00 PM	0	1	5	13	1	9	2	69	4	7	91	11
TOTAL	0	5	32	93	10	84	48	591	15	20	745	118
Peak Hour 5:00 PM - 6:00 PM	0	3	19	48	5	39	26	289	6	12	403	62

Roadway Count Summary

Vanasse Hangen Brustlin, Inc.

County

Orange

City

Orlando

Intersection

CVS

& McCulloch Rd

Date

September 8, 2022

Trucks

VHB Project #:

63289.04

AM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
7:00 AM - 7:15 AM	0	0	0	0	0	0	0	1	0	0	3	1
7:15 AM - 7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM - 7:45 AM	0	0	0	0	0	0	1	2	0	0	1	0
7:45 AM - 8:00 AM	0	0	0	0	0	0	0	0	0	0	2	0
8:00 AM - 8:15 AM	0	0	0	0	0	0	1	3	0	0	1	0
8:15 AM - 8:30 AM	0	0	0	2	0	1	1	0	0	0	2	0
8:30 AM - 8:45 AM	0	0	0	0	0	0	1	0	0	0	8	0
8:45 AM - 9:00 AM	0	0	0	0	0	0	0	4	0	0	3	1
TOTAL	0	0	0	2	0	1	4	10	0	0	20	2
Peak Hour												
7:15 AM - 8:15 AM	0	0	0	0	0	0	2	5	0	0	4	0
	0%	0%	0%	0%	0%	0%	15%	2%	0%	0%	1%	0%

PM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
4:00 PM - 4:15 PM	0	0	0	0	0	0	0	3	0	0	1	0
4:15 PM - 4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM - 4:45 PM	0	0	0	0	0	0	0	0	0	0	1	0
4:45 PM - 5:00 PM	0	0	0	0	0	0	0	0	0	0	1	1
5:00 PM - 5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM - 5:30 PM	0	0	0	0	0	1	0	0	0	0	0	0
5:30 PM - 5:45 PM	0	0	0	0	0	0	0	0	0	0	1	0
5:45 PM - 6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	0	0	0	0	1	0	3	0	0	4	1
Peak Hour												
5:00 PM - 6:00 PM	0	0	0	0	0	1	0	0	0	0	1	0
	0%	0%	0%	0%	0%	3%	0%	0%	0%	0%	0%	0%

Vanasse Hangen Brustlin, Inc.

City Orlando

& McCulloch Rd

U-Turns & RTOR

VHB Project #: 63289.04

AM Peak Hour

Time Period			Northbound			Southbound			Eastbound			Westbound		
			Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
7:00 AM	-	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	-	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	-	7:45 AM	0	0	0	0	0	0	1	0	0	0	0	0
7:45 AM	-	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	-	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	-	8:30 AM	0	0	0	0	0	0	0	0	0	1	0	0
8:30 AM	-	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	-	9:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL			0	0	0	0	0	0	1	0	0	1	0	0
Peak Hour														
7:30 AM	-	8:30 AM	0	0	0	0	0	0	1	0	0	1	0	0

PM Peak Hour

[illegible]

Pedestrian & Bicycle Summary

Project #: 63289.04

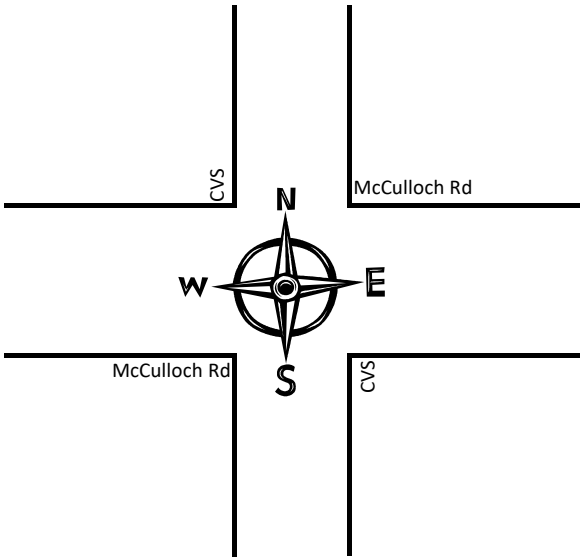
NB/SB: CVS

Date: 9/8/2022

EB/WB: McCulloch Rd

		Hour									
		7:00	8:00	11:00	12:00	14:00	15:00	16:00	17:00		
		1	2	3	4	5	6	7	8		
Eastbound	Bike	0	0	0	0	0	0	0	0	0	0
	Ped	0	1	0	0	0	0	0	0	1	1
Westbound	Bike	0	0	0	0	0	0	0	0	0	0
	Ped	0	0	0	0	0	0	2	0	2	2

		Southbound		Northbound	
Hour		Ped ▼	Bike	Ped ▲	Bike
1	7:00	0	1	0	0
2	8:00	0	0	1	0
3	11:00	0	0	0	0
4	12:00	0	0	0	0
5	14:00	0	0	0	0
6	15:00	0	0	0	0
7	16:00	1	0	0	0
8	17:00	0	0	0	0
		1	1	1	0



		Southbound		Northbound		Hour	
		Ped ▼	Bike	Ped ▲	Bike		
1	7:00	0	0	0	0	1	7:00
2	8:00	0	0	0	0	2	8:00
3	11:00	0	0	0	0	3	11:00
4	12:00	0	0	0	0	4	12:00
5	14:00	0	0	0	0	5	14:00
6	15:00	0	0	0	0	6	15:00
7	16:00	0	0	0	0	7	16:00
8	17:00	0	0	0	0	8	17:00
		0	0	0	0		

Eastbound	Bike	0	0	0	0	0	0	0	0	0	0
	Ped	1	1	0	0	0	0	0	0	2	2
Westbound	Bike	0	0	0	0	0	0	1	0	1	1
	Ped	0	1	0	0	0	0	0	0	1	1

7:00	8:00	11:00	12:00	14:00	15:00	16:00	17:00
1	2	3	4	5	6	7	8

8

2

Hour

Roadway Count Summary

Vanasse Hangen Brustlin, Inc.

County

Orange

City

Orlando

Intersection

SR 434

& McCulloch Rd

Date

September 8, 2022

All Vehicles

VHB Project #:

63289.04

AM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
7:00 AM - 7:15 AM	2	82	19	42	180	39	18	15	1	68	83	104
7:15 AM - 7:30 AM	5	90	17	65	221	59	24	21	2	83	93	98
7:30 AM - 7:45 AM	1	133	36	41	227	71	20	19	2	86	90	88
7:45 AM - 8:00 AM	3	112	26	82	301	64	40	18	4	97	84	101
8:00 AM - 8:15 AM	3	116	35	85	313	64	39	36	4	100	75	118
8:15 AM - 8:30 AM	6	129	51	98	332	59	28	27	7	91	70	114
8:30 AM - 8:45 AM	7	142	51	130	322	63	34	30	3	115	86	87
8:45 AM - 9:00 AM	8	161	48	125	256	41	43	36	6	90	74	65
TOTAL	35	965	283	668	2,152	460	246	202	29	730	655	775
Peak Hour 8:00 AM - 9:00 AM	24	548	185	438	1,223	227	144	129	20	396	305	384

PM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
4:00 PM - 4:15 PM	14	293	117	101	187	52	41	41	8	73	54	101
4:15 PM - 4:30 PM	13	277	94	92	169	45	35	30	1	59	39	78
4:30 PM - 4:45 PM	16	282	98	102	140	37	46	44	2	60	40	88
4:45 PM - 5:00 PM	8	234	111	109	175	41	36	57	3	67	43	86
5:00 PM - 5:15 PM	10	336	131	117	203	59	31	48	5	64	52	96
5:15 PM - 5:30 PM	14	352	120	125	203	55	45	46	3	82	69	105
5:30 PM - 5:45 PM	16	320	117	131	195	50	46	52	6	77	53	99
5:45 PM - 6:00 PM	15	306	124	112	152	41	33	57	2	92	50	94
TOTAL	106	2,400	912	889	1,424	380	313	375	30	574	400	747
Peak Hour 5:00 PM - 6:00 PM	55	1,314	492	485	753	205	155	203	16	315	224	394

Roadway Count Summary

Vanasse Hangen Brustlin, Inc.

County

Orange

City

Orlando

Intersection

SR 434

& McCulloch Rd

Date

September 8, 2022

Trucks

VHB Project #:

63289.04

AM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
7:00 AM - 7:15 AM	0	0	0	1	2	1	0	1	0	0	4	0
7:15 AM - 7:30 AM	0	4	0	2	3	1	0	0	0	0	0	0
7:30 AM - 7:45 AM	0	1	1	0	2	1	1	3	0	0	0	0
7:45 AM - 8:00 AM	0	10	0	4	4	0	0	1	0	0	1	0
8:00 AM - 8:15 AM	0	1	0	3	3	0	1	1	1	0	2	2
8:15 AM - 8:30 AM	0	1	2	1	5	1	0	1	1	3	1	2
8:30 AM - 8:45 AM	0	0	1	4	4	2	0	3	0	0	1	1
8:45 AM - 9:00 AM	0	2	1	4	6	2	1	2	0	1	3	1
TOTAL	0	19	5	19	29	8	3	12	2	4	12	6
Peak Hour 8:00 AM - 9:00 AM	0	4	4	12	18	5	2	7	2	4	7	6
	0%	1%	2%	3%	1%	2%	1%	5%	10%	1%	2%	2%

PM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
4:00 PM - 4:15 PM	0	1	1	2	3	0	1	1	0	1	3	3
4:15 PM - 4:30 PM	0	0	0	2	1	0	0	0	0	2	0	3
4:30 PM - 4:45 PM	0	1	1	2	3	0	0	0	0	0	1	1
4:45 PM - 5:00 PM	0	1	0	2	1	1	0	0	0	1	1	1
5:00 PM - 5:15 PM	0	0	0	2	1	0	0	0	0	0	0	1
5:15 PM - 5:30 PM	0	1	1	1	0	0	0	0	0	0	0	4
5:30 PM - 5:45 PM	0	1	0	1	3	0	0	0	0	0	0	1
5:45 PM - 6:00 PM	0	4	0	2	0	0	0	1	0	0	0	1
TOTAL	0	9	3	14	12	1	1	2	0	4	5	15
Peak Hour 5:00 PM - 6:00 PM	0	6	1	6	4	0	0	1	0	0	0	7
	0%	0%	0%	1%	1%	0%	0%	0%	0%	0%	0%	2%

Roadway Count Summary

Vanasse Hangen Brustlin, Inc.

County

Orange

City

Orlando

Intersection

SR 434

& McCulloch Rd

Date

September 8, 2022

U-Turns & RTOR

VHB Project #:

63289.04

AM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
7:00 AM - 7:15 AM	1	0	11	1	0	6	0	0	0	0	0	65
7:15 AM - 7:30 AM	0	0	5	4	0	12	0	0	0	2	0	38
7:30 AM - 7:45 AM	0	0	6	0	0	19	0	0	0	0	0	45
7:45 AM - 8:00 AM	1	0	2	2	0	16	0	0	1	0	0	39
8:00 AM - 8:15 AM	1	0	16	5	0	14	0	0	1	0	0	45
8:15 AM - 8:30 AM	1	0	19	1	0	15	0	0	3	0	0	62
8:30 AM - 8:45 AM	4	0	15	4	0	16	0	0	0	1	0	41
8:45 AM - 9:00 AM	1	0	14	6	0	15	0	0	1	0	0	23
TOTAL	9	0	88	23	0	113	0	0	6	3	0	358
Peak Hour												
7:45 AM - 8:45 AM	7	0	52	12	0	61	0	0	5	1	0	187

PM Peak Hour

Time Period	Northbound			Southbound			Eastbound			Westbound		
	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
4:00 PM - 4:15 PM	0	0	23	4	0	20	0	0	0	0	0	54
4:15 PM - 4:30 PM	3	0	34	5	0	8	0	0	0	0	0	25
4:30 PM - 4:45 PM	5	0	24	6	0	16	0	0	1	1	0	38
4:45 PM - 5:00 PM	2	0	16	4	0	18	0	0	0	0	0	45
5:00 PM - 5:15 PM	2	0	29	10	0	20	0	0	1	1	0	33
5:15 PM - 5:30 PM	1	0	37	6	0	23	0	0	1	2	0	31
5:30 PM - 5:45 PM	2	0	28	9	0	14	1	0	2	2	0	39
5:45 PM - 6:00 PM	6	0	14	5	0	15	0	0	0	0	0	44
TOTAL	21	0	205	49	0	134	1	0	5	6	0	309
Peak Hour												
4:45 PM - 5:45 PM	7	0	110	29	0	75	1	0	4	5	0	148

Pedestrian & Bicycle Summary

Project #: 63289.04

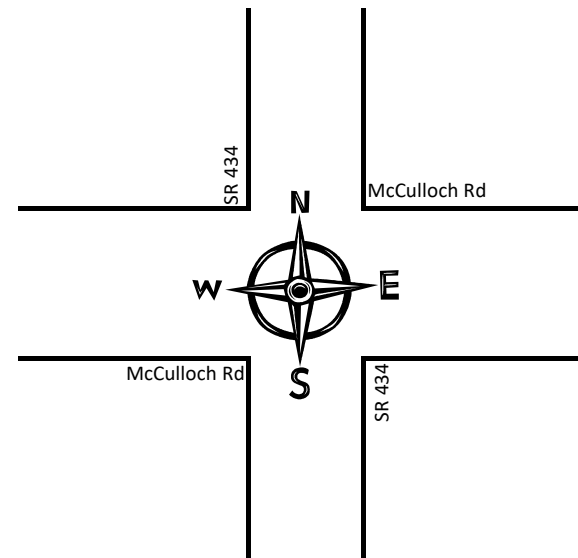
NB/SB: SR 434

Date: 9/8/2022

EB/WB: McCulloch Rd

		Hour									
		7:00	8:00	11:00	12:00	14:00	15:00	16:00	17:00		
		1	2	3	4	5	6	7	8		
Eastbound	Bike	0	1	0	0	0	0	1	0	2	3
	Ped	0	2	0	0	0	0	0	1		
Westbound	Bike	0	0	0	0	0	0	0	1	1	2
	Ped	1	0	0	0	0	0	0	1		

		Southbound		Northbound	
Hour		Ped ▼	Bike	Ped ▲	Bike
1	7:00	3	0	2	0
2	8:00	1	1	2	0
3	11:00	0	0	0	0
4	12:00	0	0	0	0
5	14:00	0	0	0	0
6	15:00	0	0	0	0
7	16:00	0	1	0	0
8	17:00	1	1	0	0
		5	3	4	0



		Southbound		Northbound		Hour	
		Ped ▼	Bike	Ped ▲	Bike		
1	7:00	0	1	0	0	1	7:00
2	8:00	1	0	1	1	2	8:00
3	11:00	0	0	0	0	3	11:00
4	12:00	0	0	0	0	4	12:00
5	14:00	0	0	0	0	5	14:00
6	15:00	0	0	0	0	6	15:00
7	16:00	0	0	0	0	7	16:00
8	17:00	2	0	1	0	8	17:00
		3	1	2	1		

Eastbound	Bike	0	0	0	0	0	0	0	0	0	0
	Ped	0	0	0	0	0	0	0	0		
Westbound	Bike	0	0	0	0	0	0	0	0	0	0
	Ped	0	0	0	0	0	0	0	0		

7:00	8:00	11:00	12:00	14:00	15:00	16:00	17:00
1	2	3	4	5	6	7	8

Appendix B: Signal Timing Plans



ORANGE COUNTY TRAFFIC SIGNAL TIMING

Intersection: Alafaya TI & McCulloch Rd
Equipment: Eagle

CDI:

CDO:

Node: 337
Date: 07/30/19

BASIC TIMING

Phase	1	2	3	4	5	6	7	8
Direction	NBL	SB	EBL	WB	SBL	NB	WBL	EB
Min Green (sec)	5	10	5	5	5	10	5	5
Vehicle Gap (sec)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Max Green 1 (sec)	15	55	15	20	15	55	25	20
Max Green 2 (sec)	15	55	15	20	15	55	25	20
Yellow (sec)	4.9	5.1	4.1	4.9	5.1	4.9	4.9	4.1
All-Red (sec)	3.3	2.0	3.0	2.0	3.1	2.0	2.8	2.2
Walk (sec)		7		7		7		
Flash Don't Walk (sec)		28		38		38		
Recall/Memory	NL	SF/NL	NL	NL	NL	SF/NL	NL	NL
Delay (sec)								
Detector Switching								
Dual Entry		Y		Y		Y		Y
Overlap								
Flash	R	Y	R	R	R	Y	R	R
Speed (mph)	45	50	35	45	50	45	45	35
Veh Distance (ft)	135	90	140	142	90	135	142	140
Ped Distance (ft)		95.0		131.0		132.0		
Ped Clearance (sec)		27		37		38		

COORDINATION PLANS

Coordination Pattern	1/1/1	2/1/1	2/2/1	3/1/1		Day	Time	Pattern
Cycle	180	160	180	190		1	0:30	FREE
Split 1	20	24	25	30		1	10:00	2/2/1
Split 2	75	56	78	82		2	0:30	FREE
Split 3	20	28	27	27		2	6:00	1/1/1
Split 4	65	52	50	51		2	9:30	2/2/1
Split 5	35	26	35	30		2	15:30	3/1/1
Split 6	60	54	68	82		2	19:00	2/1/1
Split 7	55	35	45	38				
Split 8	30	45	32	40				
Offset	102	155	103	61				
Lagging Phases	0/0/0/7	1/3/0/7	0/0/0/0	1/0/0/0				
Source Day	Equate 1	Equate 2	Equate 3	Equate 4	Equate 5			
1	7							
2	3	4	5	6				

Notes:

ALT SEQ 1 = REVERSE PHASE 1 & 2

ALT SEQ 8 = REVERSE PHASE 7 & 8

ALT SEQ 11 = REVERSE PHASE 1 & 2, 3 & 4 AND 7 & 8

1. This intersection operates under adaptive control

2. When not running adaptive control adjust Vehicle Gap to 3.0 for all phases

Appendix C:
Peak Season Factor Category
Report



2021 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL
 CATEGORY: 7700 SEMINOLE COUNTYWIDE

WEEK	DATES	SF	MOCF: 0.98 PSCF
1	01/01/2021 - 01/02/2021	1.01	1.03
2	01/03/2021 - 01/09/2021	1.04	1.06
3	01/10/2021 - 01/16/2021	1.07	1.09
4	01/17/2021 - 01/23/2021	1.06	1.08
5	01/24/2021 - 01/30/2021	1.05	1.07
6	01/31/2021 - 02/06/2021	1.04	1.06
7	02/07/2021 - 02/13/2021	1.03	1.05
8	02/14/2021 - 02/20/2021	1.02	1.04
9	02/21/2021 - 02/27/2021	1.01	1.03
10	02/28/2021 - 03/06/2021	0.99	1.01
*11	03/07/2021 - 03/13/2021	0.98	1.00
*12	03/14/2021 - 03/20/2021	0.97	0.99
*13	03/21/2021 - 03/27/2021	0.97	0.99
*14	03/28/2021 - 04/03/2021	0.97	0.99
*15	04/04/2021 - 04/10/2021	0.98	1.00
*16	04/11/2021 - 04/17/2021	0.98	1.00
*17	04/18/2021 - 04/24/2021	0.98	1.00
*18	04/25/2021 - 05/01/2021	0.98	1.00
*19	05/02/2021 - 05/08/2021	0.97	0.99
*20	05/09/2021 - 05/15/2021	0.97	0.99
*21	05/16/2021 - 05/22/2021	0.98	1.00
*22	05/23/2021 - 05/29/2021	0.98	1.00
*23	05/30/2021 - 06/05/2021	0.99	1.01
24	06/06/2021 - 06/12/2021	0.99	1.01
25	06/13/2021 - 06/19/2021	1.00	1.02
26	06/20/2021 - 06/26/2021	1.00	1.02
27	06/27/2021 - 07/03/2021	1.01	1.03
28	07/04/2021 - 07/10/2021	1.01	1.03
29	07/11/2021 - 07/17/2021	1.02	1.04
30	07/18/2021 - 07/24/2021	1.02	1.04
31	07/25/2021 - 07/31/2021	1.03	1.05
32	08/01/2021 - 08/07/2021	1.03	1.05
33	08/08/2021 - 08/14/2021	1.04	1.06
34	08/15/2021 - 08/21/2021	1.04	1.06
35	08/22/2021 - 08/28/2021	1.04	1.06
36	08/29/2021 - 09/04/2021	1.04	1.06
37	09/05/2021 - 09/11/2021	1.03	1.05
38	09/12/2021 - 09/18/2021	1.03	1.05
39	09/19/2021 - 09/25/2021	1.02	1.04
40	09/26/2021 - 10/02/2021	1.01	1.03
41	10/03/2021 - 10/09/2021	0.99	1.01
42	10/10/2021 - 10/16/2021	0.98	1.00
43	10/17/2021 - 10/23/2021	0.99	1.01
44	10/24/2021 - 10/30/2021	0.99	1.01
45	10/31/2021 - 11/06/2021	1.00	1.02
46	11/07/2021 - 11/13/2021	1.00	1.02
47	11/14/2021 - 11/20/2021	1.01	1.03
48	11/21/2021 - 11/27/2021	1.01	1.03
49	11/28/2021 - 12/04/2021	1.01	1.03
50	12/05/2021 - 12/11/2021	1.01	1.03
51	12/12/2021 - 12/18/2021	1.01	1.03
52	12/19/2021 - 12/25/2021	1.04	1.06
53	12/26/2021 - 12/31/2021	1.07	1.09

* PEAK SEASON

08-MAR-2022 12:36:27

830UPD

5_7700_PKSEASON.TXT

Appendix D:
Existing Condition Synchro
Reports



Intersection												
Int Delay, s/veh	1.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	16	245	3	2	564	33	2	0	3	38	1	18
Future Vol, veh/h	16	245	3	2	564	33	2	0	3	38	1	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	120	-	-	80	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	19	3	0	0	3	3	0	0	0	5	0	6
Mvmt Flow	18	269	3	2	620	36	2	0	3	42	1	20
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	656	0	0	272	0	0	960	967	271	950	950	638
Stage 1	-	-	-	-	-	-	307	307	-	642	642	-
Stage 2	-	-	-	-	-	-	653	660	-	308	308	-
Critical Hdwy	4.29	-	-	4.1	-	-	7.1	6.5	6.2	7.15	6.5	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.15	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.15	5.5	-
Follow-up Hdwy	2.371	-	-	2.2	-	-	3.5	4	3.3	3.545	4	3.354
Pot Cap-1 Maneuver	856	-	-	1303	-	-	238	256	773	237	262	469
Stage 1	-	-	-	-	-	-	707	665	-	458	472	-
Stage 2	-	-	-	-	-	-	460	463	-	696	664	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	856	-	-	1303	-	-	223	250	773	232	256	469
Mov Cap-2 Maneuver	-	-	-	-	-	-	223	250	-	232	256	-
Stage 1	-	-	-	-	-	-	692	651	-	448	471	-
Stage 2	-	-	-	-	-	-	439	462	-	678	650	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.6			0			9.7			20.5		
HCM LOS							A			C		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2			
Capacity (veh/h)	773	856	-	-	1303	-	-	233	469			
HCM Lane V/C Ratio	0.004	0.021	-	-	0.002	-	-	0.184	0.042			
HCM Control Delay (s)	9.7	9.3	-	-	7.8	-	-	23.9	13			
HCM Lane LOS	A	A	-	-	A	-	-	C	B			
HCM 95th %tile Q(veh)	0	0.1	-	-	0	-	-	0.7	0.1			

HCM 6th Signalized Intersection Summary

2: SR 434 & McCulloch Road

10/07/2022

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	148	133	21	408	314	396	25	564	191	451	1260	234
Future Volume (veh/h)	148	133	21	408	314	396	25	564	191	451	1260	234
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1697	1670	1617	1697	1697	1697	1710	1710	1697	1683	1697	1697
Adj Flow Rate, veh/h	157	141	17	434	334	345	27	600	135	480	1340	185
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	1	3	7	1	1	1	0	0	1	2	1	1
Cap, veh/h	193	160	19	666	434	368	34	1614	497	527	2291	711
Arrive On Green	0.06	0.11	0.11	0.21	0.26	0.26	0.02	0.35	0.35	0.17	0.49	0.49
Sat Flow, veh/h	3135	1462	176	3135	1697	1438	1629	4668	1438	3110	4632	1438
Grp Volume(v), veh/h	157	0	158	434	334	345	27	600	135	480	1340	185
Grp Sat Flow(s),veh/h/ln	1567	0	1638	1567	1697	1438	1629	1556	1438	1555	1544	1438
Q Serve(g_s), s	8.9	0.0	17.1	22.8	32.8	42.3	3.0	17.4	6.7	27.3	37.0	13.4
Cycle Q Clear(g_c), s	8.9	0.0	17.1	22.8	32.8	42.3	3.0	17.4	6.7	27.3	37.0	13.4
Prop In Lane	1.00		0.11	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	193	0	179	666	434	368	34	1614	497	527	2291	711
V/C Ratio(X)	0.81	0.00	0.88	0.65	0.77	0.94	0.81	0.37	0.27	0.91	0.58	0.26
Avail Cap(c_a), veh/h	266	0	283	666	476	403	71	1614	497	658	2291	711
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	83.4	0.0	79.0	64.8	62.0	65.5	87.8	44.2	12.8	73.4	32.3	26.4
Incr Delay (d2), s/veh	12.6	0.0	17.3	2.3	6.9	28.2	34.5	0.7	1.3	14.6	1.1	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	7.2	0.0	12.8	14.4	21.5	25.5	2.9	11.3	4.4	17.7	20.5	8.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	96.1	0.0	96.3	67.0	68.9	93.8	122.3	44.9	14.2	88.0	33.4	27.3
LnGrp LOS	F	A	F	E	E	F	F	D	B	F	C	C
Approach Vol, veh/h	315			1113			762			2005		
Approach Delay, s/veh	96.2			75.9			42.2			45.9		
Approach LOS	F			E			D			D		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.9	96.1	18.2	53.8	38.7	69.3	46.0	26.0				
Change Period (Y+Rc), s	* 8.2	7.1	7.1	* 7.7	8.2	* 7.1	* 7.7	* 6.3				
Max Green Setting (Gmax), s	* 7.8	77.1	15.3	* 51	38.1	* 47	* 35	* 31				
Max Q Clear Time (g_c+I1), s	5.0	39.0	10.9	44.3	29.3	19.4	24.8	19.1				
Green Ext Time (p_c), s	0.0	14.3	0.2	1.8	1.2	5.0	1.2	0.6				

Intersection Summary

HCM 6th Ctrl Delay	57.0
HCM 6th LOS	E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM 6th TWSC
1: Private Roadway & McCulloch Road

10/07/2022

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	27	298	6	12	415	64	0	3	20	49	5	40
Future Vol, veh/h	27	298	6	12	415	64	0	3	20	49	5	40
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	120	-	-	80	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	3
Mvmt Flow	29	324	7	13	451	70	0	3	22	53	5	43

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	521	0	0	331	0	0	-	933	328	910	901	486
Stage 1	-	-	-	-	-	-	-	386	-	512	512	-
Stage 2	-	-	-	-	-	-	-	547	-	398	389	-
Critical Hdwy	4.1	-	-	4.1	-	-	-	6.5	6.2	7.1	6.5	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	-	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	-	4	3.3	3.5	4	3.327
Pot Cap-1 Maneuver	1056	-	-	1240	-	-	0	268	718	258	280	579
Stage 1	-	-	-	-	-	-	0	614	-	548	540	-
Stage 2	-	-	-	-	-	-	0	521	-	632	612	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1056	-	-	1240	-	-	-	258	718	241	270	579
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	258	-	241	270	-
Stage 1	-	-	-	-	-	-	-	597	-	533	535	-
Stage 2	-	-	-	-	-	-	-	516	-	593	595	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			0.2			11.5			19.1		
HCM LOS							B			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	583	1056	-	-	1240	-	-	243	579
HCM Lane V/C Ratio	0.043	0.028	-	-	0.011	-	-	0.242	0.075
HCM Control Delay (s)	11.5	8.5	-	-	7.9	-	-	24.5	11.7
HCM Lane LOS	B	A	-	-	A	-	-	C	B
HCM 95th %tile Q(veh)	0.1	0.1	-	-	0	-	-	0.9	0.2

HCM 6th Signalized Intersection Summary

2: SR 434 & McCulloch Road

10/07/2022

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	160	209	16	324	231	406	57	1353	507	500	776	211
Future Volume (veh/h)	160	209	16	324	231	406	57	1353	507	500	776	211
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1710	1710	1710	1710	1710	1697	1710	1710	1710	1697	1710	1710
Adj Flow Rate, veh/h	168	220	13	341	243	272	60	1424	420	526	817	146
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	0	0	0	0	1	0	0	0	1	0	0
Cap, veh/h	200	239	14	376	356	299	182	1816	564	563	2160	670
Arrive On Green	0.06	0.15	0.15	0.12	0.21	0.21	0.11	0.39	0.39	0.18	0.46	0.46
Sat Flow, veh/h	3159	1599	94	3159	1710	1438	1629	4668	1449	3135	4668	1449
Grp Volume(v), veh/h	168	0	233	341	243	272	60	1424	420	526	817	146
Grp Sat Flow(s),veh/h/ln	1580	0	1693	1580	1710	1438	1629	1556	1449	1567	1556	1449
Q Serve(g_s), s	10.0	0.0	25.8	20.3	24.9	35.1	6.5	51.0	47.4	31.4	21.7	8.5
Cycle Q Clear(g_c), s	10.0	0.0	25.8	20.3	24.9	35.1	6.5	51.0	47.4	31.4	21.7	8.5
Prop In Lane	1.00		0.06	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	200	0	253	376	356	299	182	1816	564	563	2160	670
V/C Ratio(X)	0.84	0.00	0.92	0.91	0.68	0.91	0.33	0.78	0.75	0.93	0.38	0.22
Avail Cap(c_a), veh/h	219	0	297	414	405	341	182	1816	564	607	2160	670
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	88.0	0.0	79.7	82.7	69.5	73.5	77.8	51.0	49.9	76.8	33.3	16.8
Incr Delay (d2), s/veh	22.5	0.0	30.1	22.2	4.0	25.5	1.0	3.5	8.7	20.9	0.5	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	8.3	0.0	19.5	14.6	17.0	21.6	5.0	28.1	25.8	20.6	13.3	5.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	110.5	0.0	109.8	104.9	73.4	99.0	78.9	54.5	58.6	97.8	33.8	17.6
LnGrp LOS	F	A	F	F	E	F	E	D	E	F	C	B
Approach Vol, veh/h	401			856			1904			1489		
Approach Delay, s/veh	110.1			94.1			56.2			54.8		
Approach LOS	F			F			E			D		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	29.4	95.0	19.2	46.4	42.3	82.1	30.3	35.3				
Change Period (Y+Rc), s	8.2	* 7.1	7.1	6.9	8.2	* 8.2	* 7.7	* 6.9				
Max Green Setting (Gmax), s	14.6	* 88	13.2	45.0	36.8	* 66	* 25	* 33				
Max Q Clear Time (g_c+I1), s	8.5	23.7	12.0	37.1	33.4	53.0	22.3	27.8				
Green Ext Time (p_c), s	0.0	7.7	0.1	1.5	0.7	9.0	0.3	0.6				

Intersection Summary

HCM 6th Ctrl Delay 67.4

HCM 6th LOS E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Appendix E: Crash Data

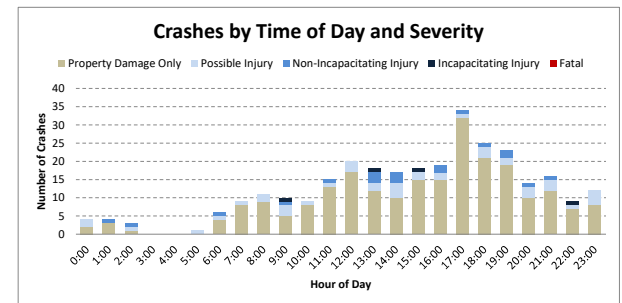
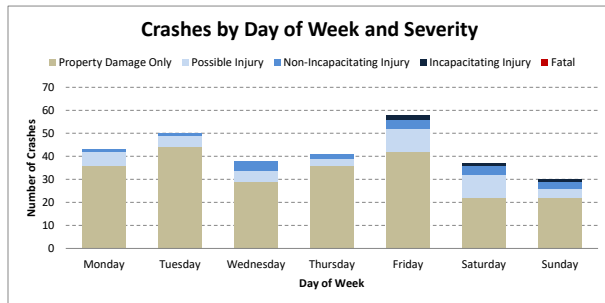
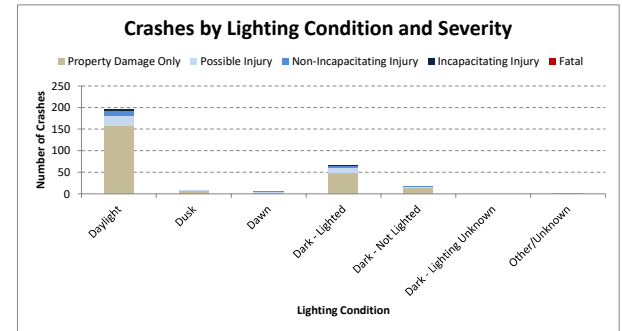
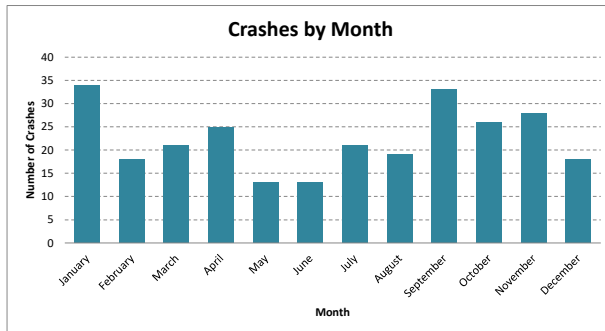
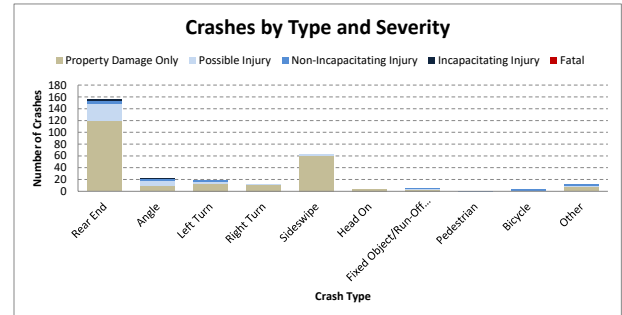
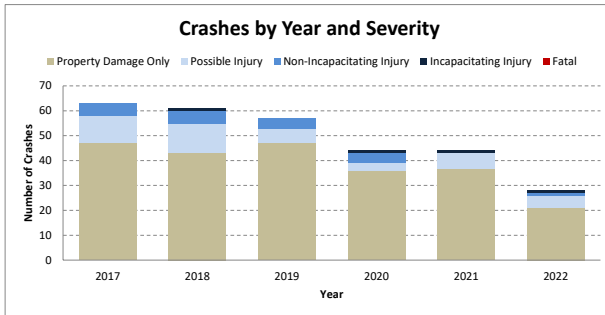


CRASH ANALYSIS - SR 434 at McCulloch Road and Surrounding Area

		Analysis Year*						Severity					Total	Annual Average*	Percent*
		2017	2018	2019	2020	2021	2022	Property Damage Only	Possible Injury	Non-Incapacitating Injury	Incapacitating Injury	Fatal			
Type of Crash	Rear End	34	28	34	23	27	11	120	28	6	3	0	157	29.2	54.3%
	Angle	6	8	4	1	1	3	10	7	5	1	0	23	4.0	7.4%
	Left Turn	3	7	0	4	1	3	14	2	2	0	0	18	3.0	5.6%
	Right Turn	2	2	2	0	3	3	11	1	0	0	0	12	1.8	3.3%
	Sideswipe	13	12	13	8	11	6	60	3	0	0	0	63	11.4	21.2%
	Head On	1	0	1	2	0	0	4	0	0	0	0	4	0.8	1.5%
	Fixed Object/Run-Off Road	1	1	1	1	0	1	3	1	1	0	0	5	0.8	1.5%
	Pedestrian	0	0	0	0	0	1	0	0	1	0	0	1	0.0	0.0%
	Bicycle	0	0	1	2	0	0	1	0	2	0	0	3	0.6	1.1%
	Other	3	3	1	3	1	0	8	1	2	0	0	11	2.2	4.1%
Total Crashes		63	61	57	44	44	28	231	43	19	4	0	297	53.8	100.0%
Crash Severity	Property Damage Only	47	43	47	36	37	21						231	42.0	78.1%
	Possible Injury	11	12	6	3	6	5						43	7.6	14.1%
	Non-Incapacitating Injury	5	5	4	4	0	1						19	3.6	6.7%
	Incapacitating Injury	0	1	0	1	1	1						4	0.6	1.1%
Fatal		0	0	0	0	0	0						0	0.0	0.0%
Light Conditions	Daylight	42	35	39	30	32	19	158	24	12	3	0	197	35.6	66.2%
	Dusk	3	3	0	2	0	0	6	2	0	0	0	8	1.6	3.0%
	Dawn	1	0	2	1	1	1	3	1	2	0	0	6	1.0	1.9%
	Dark - Lighted	10	19	10	10	9	8	49	13	3	1	0	66	11.6	21.6%
	Dark - Not Lighted	7	4	6	1	1	0	14	3	2	0	0	19	3.8	7.1%
	Dark - Lighting Unknown	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0%
	Other/Unknown	0	0	0	0	1	0	1	0	0	0	0	1	0.2	0.4%
Road Surface Condition	Dry	58	42	47	32	32	23	180	35	15	4	0	234	42.2	78.4%
	Wet	5	19	10	12	12	5	51	8	4	0	0	63	11.6	21.6%
	Other	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0%
Month	January	8	8	8	8	2	5						39	6.8	12.6%
	February	4	5	0	6	3	2						20	3.6	6.7%
	March	2	5	9	3	2	3						24	4.2	7.8%
	April	10	4	7	0	4	8						33	5.0	9.3%
	May	3	5	1	0	4	4						17	2.6	4.8%
	June	2	4	4	1	2	3						16	2.6	4.8%
	July	5	3	5	3	5	0						21	4.2	7.8%
	August	6	4	4	1	4	3						22	3.8	7.1%
	September	5	9	3	6	10	0						33	6.6	12.3%
	October	3	5	6	8	4	0						26	5.2	9.7%
	November	8	6	6	5	3	0						28	5.6	10.4%
	December	7	3	4	3	1	0						18	3.6	6.7%
Day of Week	Monday	10	12	6	5	7	3	36	6	1	0	0	43	8.0	14.9%
	Tuesday	13	13	7	6	6	5	44	5	1	0	0	50	9.0	16.7%
	Wednesday	6	8	12	6	4	2	29	5	4	0	0	38	7.2	13.4%
	Thursday	7	5	10	7	6	6	36	3	2	0	0	41	7.0	13.0%
	Friday	12	9	12	10	10	5	42	10	4	2	0	58	10.6	19.7%
	Saturday	9	7	5	4	7	5	22	10	4	1	0	37	6.4	11.9%
	Sunday	6	7	5	6	4	2	22	4	3	1	0	30	5.6	10.4%
Hour of Day	0:00	1	3	0	0	0	0	2	2	0	0	0	4	0.8	1.5%
	1:00	1	0	2	1	0	0	3	0	1	0	0	4	0.8	1.5%
	2:00	2	1	0	0	0	0	1	1	1	0	0	3	0.6	1.1%
	3:00	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0%
	4:00	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0%
	5:00	1	0	0	0	0	0	0	1	0	0	0	1	0.2	0.4%
	6:00	0	0	2	2	1	1	4	1	1	0	0	6	1.0	1.9%
	7:00	2	1	3	1	1	1	8	1	0	0	0	9	1.6	3.0%
	8:00	2	4	3	2	0	0	9	2	0	0	0	11	2.2	4.1%
	9:00	2	1	2	4	0	1	5	3	1	1	0	10	1.8	3.3%
	10:00	2	0	3	2	1	1	8	1	0	0	0	9	1.6	3.0%
	11:00	4	3	1	1	5	1	13	1	1	0	0	15	2.8	5.2%
	12:00	5	4	3	3	3	2	17	3	0	0	0	20	3.6	6.7%
	13:00	3	3	0	5	4	3	12	2	3	1	0	18	3.0	5.6%
	14:00	4	2	5	4	1	1	10	4	3	0	0	17	3.2	5.9%
	15:00	5	3	2	3	2	3	15	2	0	1	0	18	3.0	5.6%
	16:00	4	4	7	0	2	2	15	2	2	0	0	19	3.4	6.3%
	17:00	12	5	7	1	8	1	32	1	1	0	0	34	6.6	12.3%
	18:00	2	8	2	6	4	3	21	3	1	0	0	25	4.4	8.2%
	19:00	4	5	4	5	3	2	19	2	2	0	0	23	4.2	7.8%
	20:00	2	5	4	0	1	2	10	3	1	0	0	14	2.4	4.5%
	21:00	1	4	3	3	4	1	12	3	1	0	0	16	3.0	5.6%
	22:00	2	1	2	1	2	1	7	1	0	1	0	9	1.6	3.0%
	23:00	2	4	2	0	2	2	8	4	0	0	0	12	2.0	3.7%
Time Period	12AM-6AM	5	4	2	1	0	0	6	4	2	0	0	12	2.4	4.5%
	6AM-12PM	12	9	14	12	8	5	47	9	3	1	0	60	11.0	20.4%
	12PM-6PM	33	21	24	16	20	12	101	14	9	2	0	126	22.8	42.4%
	6PM-12AM	13	27	17	15	16	11	77	16	5	1	0	99	17.6	32.7%
Alcohol & Drugs	None	62	60	57	43	44	28	229	43	18	4	0	294	53.2	98.9%
	Alcohol Involved	0	1	0	0	0	0	1	0	0	0	0	1	0.2	0.4%
	Drugs Involved	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0%
	Alcohol and Drugs	1	0	0	1	0	0	1	0	1	0	0	2	0.4	0.7%
	Undetermined	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0%
Age of Driver 1 (Typically Driver at Fault)	19 and Under	4	10	6	5	0	0						25	5.0	9.3%
	20-24	13	8	11	13	0	0						45	9.0	16.7%
	25-29	3	7	4	2	1	0						17	3.4	6.3%
	30-34	5	4	0	2	0	0						11	2.2	4.1%
	35-39	2	0	3	1	0	0						6	1.2	2.2%
	40-44	1	3	0	3	0	0						7	1.4	2.6%
	45-49	0	1	0	0	0	1						2	0.2	0.4%
	50-54	0	0	0	2	0	0						2	0.4	0.7%
	55-59	1	1	2	2	0	0						6	1.2	2.2%
	60-64	2	0	1	1	0	0						4	0.8	1.5%
	65-69	0	0	0	0	0	0						0	0.0	0.0%
	70-74	1	0	1	1	0	0						3	0.6	1.1%
	75-79	0	0	1	0	0	0						1	0.2	0.4%
	80-84	1	1	0	0	0	0						2	0.4	0.7%
	85 and Over	0	0	1	0	0	0						1	0.2	0.4%
	Unknown	2	4	5	2	0	0						13	2.6	4.8%

*Crash data for the current year (2022) is incomplete as it only includes data through September 06, 2022; therefore, it is not included in the average crashes per year or percent calculations, but is included in the crash totals.

CRASH ANALYSIS - SR 434 at McCulloch Road and Surrounding Area



Appendix F:
Alternative Condition Synchro
Reports



HCM 6th TWSC
1: Private Roadway & McCulloch Road

10/07/2022

Intersection												
Int Delay, s/veh	0.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↰			↰				↱			↱
Traffic Vol, veh/h	0	261	3	0	564	33	0	0	5	0	0	18
Future Vol, veh/h	0	261	3	0	564	33	0	0	5	0	0	18
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	0	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	19	3	0	0	3	3	0	0	0	5	0	6
Mvmt Flow	0	287	3	0	620	36	0	0	5	0	0	20
Major/Minor	Major1		Major2		Minor1		Minor2					
Conflicting Flow All	-	0	0	-	-	0	-	-	289	-	-	638
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-	-	-	6.2	-	-	6.26
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-	-	-	3.3	-	-	3.354
Pot Cap-1 Maneuver	0	-	-	0	-	-	0	0	755	0	0	469
Stage 1	0	-	-	0	-	-	0	0	-	0	0	-
Stage 2	0	-	-	0	-	-	0	0	-	0	0	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	-	-	-	-	-	-	-	-	755	-	-	469
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	0		0		9.8		13					
HCM LOS					A		B					
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT	WBR	SBLn1						
Capacity (veh/h)	755	-	-	-	-	469						
HCM Lane V/C Ratio	0.007	-	-	-	-	0.042						
HCM Control Delay (s)	9.8	-	-	-	-	13						
HCM Lane LOS	A	-	-	-	-	B						
HCM 95th %tile Q(veh)	0	-	-	-	-	0.1						

HCM 6th Signalized Intersection Summary

2: SR 434 & McCulloch Road

10/07/2022

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	164	135	21	409	315	396	25	564	191	486	1265	233
Future Volume (veh/h)	164	135	21	409	315	396	25	564	191	486	1265	233
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1697	1670	1617	1697	1697	1697	1710	1710	1697	1683	1697	1697
Adj Flow Rate, veh/h	174	144	17	435	335	239	27	600	135	517	1346	184
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	1	3	7	1	1	1	0	0	1	2	1	1
Cap, veh/h	210	163	19	555	368	312	34	1715	528	565	2448	760
Arrive On Green	0.07	0.11	0.11	0.18	0.22	0.22	0.02	0.37	0.37	0.18	0.53	0.53
Sat Flow, veh/h	3135	1466	173	3135	1697	1438	1629	4668	1438	3110	4632	1438
Grp Volume(v), veh/h	174	0	161	435	335	239	27	600	135	517	1346	184
Grp Sat Flow(s),veh/h/ln	1567	0	1639	1567	1697	1438	1629	1556	1438	1555	1544	1438
Q Serve(g_s), s	9.9	0.0	17.4	23.9	34.7	28.1	3.0	16.8	7.0	29.4	34.8	12.5
Cycle Q Clear(g_c), s	9.9	0.0	17.4	23.9	34.7	28.1	3.0	16.8	7.0	29.4	34.8	12.5
Prop In Lane	1.00		0.11	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	210	0	182	555	368	312	34	1715	528	565	2448	760
V/C Ratio(X)	0.83	0.00	0.88	0.78	0.91	0.77	0.81	0.35	0.26	0.92	0.55	0.24
Avail Cap(c_a), veh/h	277	0	275	606	463	392	71	1715	528	688	2448	760
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	83.0	0.0	78.9	70.8	68.8	66.2	87.8	41.3	13.8	72.3	28.2	22.9
Incr Delay (d2), s/veh	14.6	0.0	19.3	6.2	19.2	6.9	34.5	0.6	1.2	15.1	0.9	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	7.9	0.0	13.2	15.4	23.9	16.4	2.9	10.9	4.5	18.9	19.3	8.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	97.5	0.0	98.2	76.9	88.0	73.1	122.3	41.9	15.0	87.4	29.1	23.7
LnGrp LOS	F	A	F	E	F	E	F	D	B	F	C	C
Approach Vol, veh/h	335		1009				762		2047			
Approach Delay, s/veh	97.8		79.7				40.0		43.3			
Approach LOS	F		E				D		D			
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	11.9	102.2	19.2	46.7	40.9	73.2	39.6	26.3				
Change Period (Y+Rc), s	* 8.2	7.1	7.1	* 7.7	8.2	* 7.1	* 7.7	* 6.3				
Max Green Setting (Gmax), s	* 7.8	77.9	15.9	* 49	39.8	* 46	* 35	* 30				
Max Q Clear Time (g_c+I1), s	5.0	36.8	11.9	36.7	31.4	18.8	25.9	19.4				
Green Ext Time (p_c), s	0.0	14.7	0.2	2.3	1.3	4.9	1.1	0.6				
Intersection Summary												
HCM 6th Ctrl Delay			55.9									
HCM 6th LOS			E									
Notes												

HCM 6th TWSC
1: Private Roadway & McCulloch Road

10/07/2022

Intersection												
Int Delay, s/veh	0.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↰			↰				↱			↱
Traffic Vol, veh/h	0	325	6	0	415	64	0	0	23	0	0	40
Future Vol, veh/h	0	325	6	0	415	64	0	0	23	0	0	40
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	0	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0	0	0	0	0	0	3
Mvmt Flow	0	353	7	0	451	70	0	0	25	0	0	43
Major/Minor	Major1		Major2			Minor1			Minor2			
Conflicting Flow All	-	0	0	-	-	0	-	-	357	-	-	486
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	-	-	-	6.2	-	-	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	-	-	-	3.3	-	-	3.327
Pot Cap-1 Maneuver	0	-	-	0	-	-	0	0	692	0	0	579
Stage 1	0	-	-	0	-	-	0	0	-	0	0	-
Stage 2	0	-	-	0	-	-	0	0	-	0	0	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	-	-	-	-	-	-	-	-	692	-	-	579
Mov Cap-2 Maneuver	-	-	-	-	-	-	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-	-	-	-	-	-	-
Approach	EB		WB			NB			SB			
HCM Control Delay, s	0		0			10.4			11.7			
HCM LOS						B			B			
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT	WBR	SBLn1						
Capacity (veh/h)	692	-	-	-	-	579						
HCM Lane V/C Ratio	0.036	-	-	-	-	0.075						
HCM Control Delay (s)	10.4	-	-	-	-	11.7						
HCM Lane LOS	B	-	-	-	-	B						
HCM 95th %tile Q(veh)	0.1	-	-	-	-	0.2						

HCM 6th Signalized Intersection Summary

2: SR 434 & McCulloch Road

10/07/2022

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	190	209	16	330	225	406	57	1353	507	547	788	206
Future Volume (veh/h)	190	209	16	330	225	406	57	1353	507	547	788	206
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1710	1710	1710	1710	1710	1697	1710	1710	1710	1697	1710	1710
Adj Flow Rate, veh/h	200	220	13	347	237	272	60	1424	420	576	829	141
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	0	0	0	0	1	0	0	0	1	0	0
Cap, veh/h	231	245	14	381	349	293	179	1718	533	611	2143	665
Arrive On Green	0.07	0.15	0.15	0.12	0.20	0.20	0.11	0.37	0.37	0.19	0.46	0.46
Sat Flow, veh/h	3159	1599	94	3159	1710	1438	1629	4668	1449	3135	4668	1449
Grp Volume(v), veh/h	200	0	233	347	237	272	60	1424	420	576	829	141
Grp Sat Flow(s),veh/h/ln	1580	0	1693	1580	1710	1438	1629	1556	1449	1567	1556	1449
Q Serve(g_s), s	11.9	0.0	25.7	20.6	24.3	35.3	6.5	52.7	49.0	34.4	22.2	8.1
Cycle Q Clear(g_c), s	11.9	0.0	25.7	20.6	24.3	35.3	6.5	52.7	49.0	34.4	22.2	8.1
Prop In Lane	1.00		0.06	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	231	0	259	381	349	293	179	1718	533	611	2143	665
V/C Ratio(X)	0.87	0.00	0.90	0.91	0.68	0.93	0.34	0.83	0.79	0.94	0.39	0.21
Avail Cap(c_a), veh/h	231	0	301	417	405	341	179	1718	533	640	2143	665
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	87.1	0.0	79.0	82.5	69.9	74.2	78.2	54.6	53.4	75.4	33.8	16.3
Incr Delay (d2), s/veh	27.2	0.0	25.4	22.5	3.7	28.6	1.1	4.8	11.2	22.1	0.5	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	9.7	0.0	19.1	14.8	16.6	21.9	5.0	29.2	26.9	22.4	13.6	5.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	114.4	0.0	104.3	105.0	73.6	102.8	79.3	59.4	64.6	97.5	34.3	17.0
LnGrp LOS	F	A	F	F	E	F	E	E	E	F	C	B
Approach Vol, veh/h	433			856			1904			1546		
Approach Delay, s/veh	109.0			95.6			61.2			56.3		
Approach LOS	F			F			E			E		
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	29.0	94.3	21.0	45.7	45.2	78.1	30.6	36.0				
Change Period (Y+Rc), s	8.2	* 7.1	7.1	6.9	8.2	* 8.2	* 7.7	* 6.9				
Max Green Setting (Gmax), s	14.6	* 87	13.9	45.0	38.8	* 63	* 25	* 34				
Max Q Clear Time (g_c+I1), s	8.5	24.2	13.9	37.3	36.4	54.7	22.6	27.7				
Green Ext Time (p_c), s	0.0	7.8	0.0	1.5	0.6	6.4	0.3	0.6				

Intersection Summary

HCM 6th Ctrl Delay 70.2

HCM 6th LOS E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.